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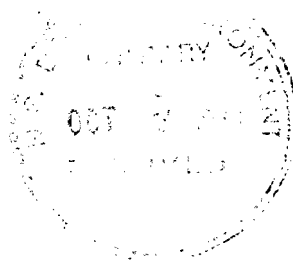
GROUNDWATER DEVELOPMENT
ANNAPOLIS VALLEY REGIONAL INDUSTRIAL PARK
KENTVILLE, NOVA SCOTIA

FINAL REPORT

SEPT 1978

D. M. CALLAN, P. Eng.
Hydrogeologist

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CONTENTS

INTRODUCTION

RESULTS OF TESTING 12" PRODUCTION WELL, SITE 2

DISCUSSION OF TEST RESULTS

WATER QUALITY

ADDITIONAL WORKS

CONCLUSIONS & RECOMMENDATIONS

FIGURES & TABLES

- Figure 1. Location Map
- Figure 2. Well Construction
- Figure 3. Time-drawdown Data Obs Well No. 2 (r = 700')
- Figure 4. Distance-drawdown Data (r = 70; r = 700)

APPENDIX

- Water Quality Analyses.
- Field Drawdown with Time Data.
- Mutual Interference Calculations.

INTRODUCTION:

This report summarizes final works undertaken in the development of groundwater supply sources for the Industrial Park.

In general overview, the groundwater potential of bedrock sources in the region of the Park was investigated in Phase 1 and summarized in a report submitted in June 1977. This investigation indicated good quality groundwater was available in a confined aquifer in Triassic Wolfville Fm sandstones at Site 1. A safe, per-well yield of about 150 igpm was assigned for an open-bore hole, unscreened well. The friable nature of the aquifer materials indicated that screens and formation-stabilizing gravels would be required to consider higher pumping rates.

The thick, permeable, saturated overburden present at Site 2 was investigated in Phase 2 and summarized in a report submitted in November 1977. In this investigation a test-production well was constructed at Site 2 and assigned a safe pumping rate of 333 igpm based on recommended entrance velocities to the screen as constructed.

In the phase 2 report, it was recommended that a higher capacity well be constructed and that the overburden aquifer be monitored on a continuing basis to provide information for possible additional development of the aquifer in the future.

An interim letter-report was submitted April 20, 1978 summarizing the construction of a 12" production well at Site 2 and the results of step-drawdown testing at rates up to 333 igpm. Final testing at higher rates was withheld until the low-recharge summer-months and the installation of a constant-measuring water-level instrument.

The present report summarizes the higher-rate testing carried out in late August 1978 and additional works including abandonment of test holes and instrumentation of an observation well at Site 1. It includes recommendations for future groundwater development in this very promising aquifer.

RESULTS OF TESTING 12" PRODUCTION WELL, SITE 2

The well is constructed as shown in Figure 1. The twenty feet of 80 slot screen installed has a transmitting capacity of about 1000 usgpm (833 igpm).

The well represents one of the two water supply sources to the Park. The rated capacity of the other well (Test-Production Well, Site 2) is 400 usgpm (333 igpm).

If full standby capability is considered, the maximum pumping rate of the 12" well will necessarily be restricted to the yield of the lower capacity test production well, or 400 usgpm (333 igpm).

The 12" well was appraised by step-drawdown testing up to rates of 400 usgpm in March 1978 using a test pump which could handle these rates.

In August, 1978, a larger test pump was installed to permit pumping at rates up to 900 usgpm.

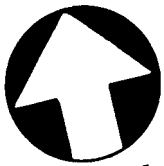
The 900 usgpm rate was continued as a constant rate test for a period of 72 hours.

The results of testing over the total range are summarized below. The duration of each step was 30 minutes.

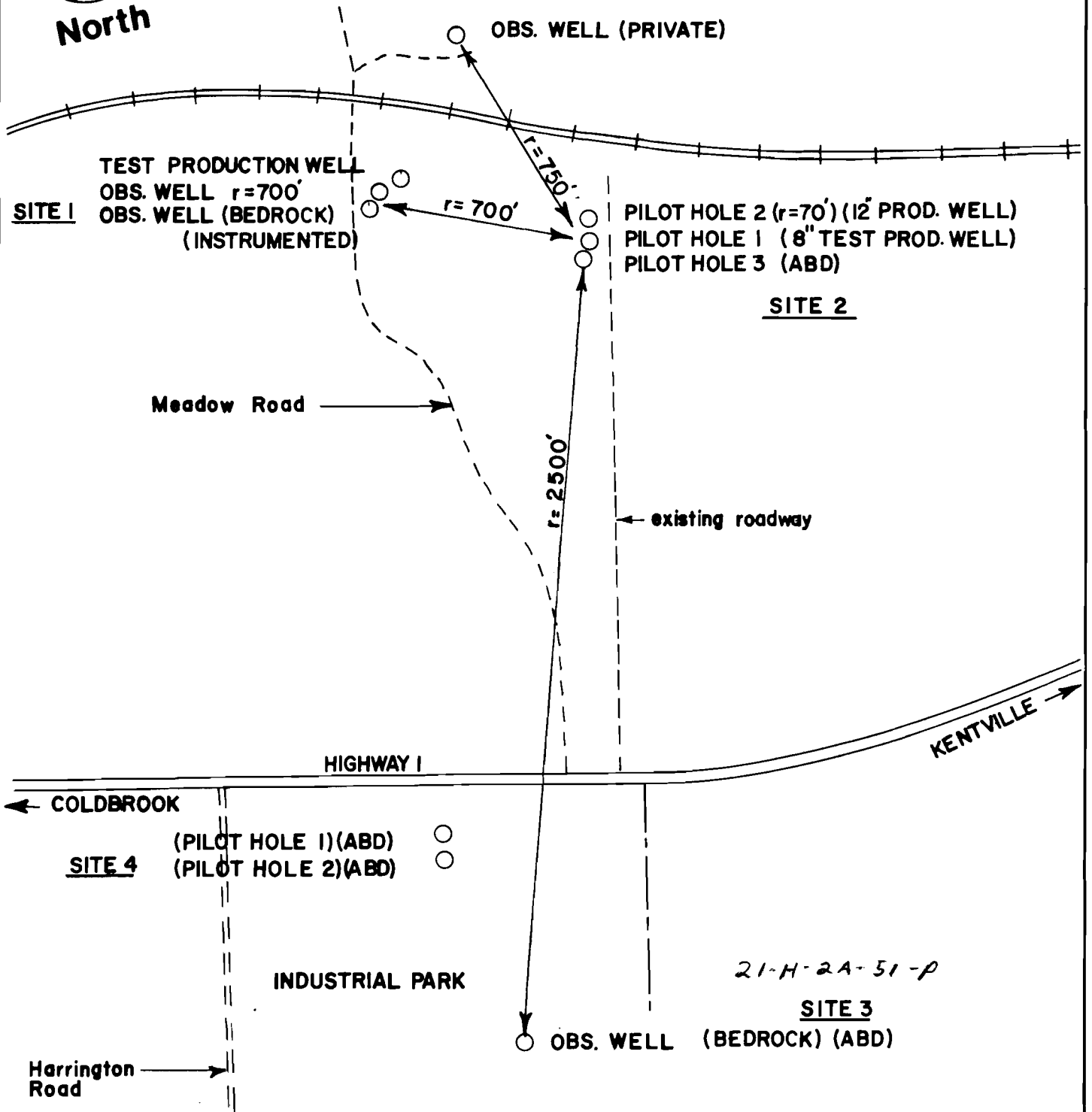
	Q (usgpm)	S (feet)	Q/S (usgpm/ft)
STEP 1	150	3.2	47
STEP 2	300	4.11	73
STEP 3	400	5.65	71
STEP 4	450	6.63	68
STEP 5	650	10.42	62
STEP 6	900	15.26	59

LOCATION SKETCH

SHOWING TEST, OBSERVATION AND PRODUCTION WELLS,
ANNAPOLIS VALLEY INDUSTRIAL PARK - KENTVILLE, N.S.



North



Scale: 1" = 400'

FIGURE 1

DISCUSSION OF TEST RESULTS

The performance of the observation wells indicates that this aquifer has excellent water-bearing characteristics. The performance of the pumped well shows formational losses are more significant than well losses. It is interpreted from this that the aquifer has relatively higher horizontal permeability than vertical permeability.

The pumped well appears to show the influence of recharge after approximately 3 hours pumping. This may in part be due to pump discharge re-entering the ground at some distance from the well however natural discharge occurs within approximately 150 feet of the pumped well to the north. Bogs and lakes in this area are accepted as representing free water table.

Both observation wells showed continuing trends of drawdown with time. Late data conforms to Theis non-leaky curves after the influences of gravity drainage are reduced.

Late drawdown and distance drawdown data probably provide the best estimation of hydraulic properties.

The calculation of hydraulic properties is shown on figures 2 and 3.

The aquifer appears to have a Transmissivity of 500,000 usgpd per foot and a storativity of 0.1.

With abundant recharge apparently available in the high water table alluvium to the north of Site 2, the maximum yield from this aquifer could be very high indeed.

Hydraulic properties calculated from the test provide estimates of interference drawdown as illustrated in the theoretical distance-drawdown curves included in the Appendix.

Anticipated interference drawdowns at 400 and 750 usgpm are shown in the Appendix. These rates represent the normal anticipated pumping draft and the maximum pumping rate of the 12" well respectively.

It is assumed in the calculations that no hydraulic barriers are intersected and that no natural precipitation occurs for a period of two years.

Discussion of Test Results (Cont'd)

At a pumping rate of 400 usgpm (333 igpm) the interference drawdown at the second well at Site 2 ($r=70'$) will be approximately 1 foot. At a distance of one half mile the water level will be lowered by 0.5 ft. and at one mile by 0.3 feet.

The hydraulic gradient induced by pumping at 400 usgpm is therefore not anticipated to have a serious impact on the existing groundwater regime.

At the higher pumping rate of 900 usgpm (750 igpm) interference drawdowns will be more pronounced reaching 2 feet at a distance of 70 feet and approximately 1 foot at half a mile.

The impact of pumping at this rate may influence natural discharge in the region of Site 2 and could interfere with other uses of water such as migratory wild fowl.

At 900 usgpm a hydraulic gradient will be induced toward the pumping centre with groundwater levels lowered by over half a foot at a distance of a mile. This possibly could, in the long term, tend to reverse gradients as far away as the Scotian Gold Apple Plant where Environment Nova Scotia note deteriorated groundwater quality.

The most practical means of establishing potential degradation of water quality resulting from groundwater gradient reversals is to monitor water levels on a continuing basis.

It is essential that the measurements be continuous in order to identify small changes due to pumping draft in the normal annual fluctuations.

For this reason a Stevens recorder was installed at Site 1, some 700 feet west of the Site 2 pumping station to monitor the impact of pumping on water levels. Specific measures for future monitoring are outlined in the recommendations.

WATER QUALITY

The water is acceptable bacteriologically; two samples collected during the sustained pumping test were rated Grade A by Victoria General Hospital Department of Microbiology. Analyses are included in the Appendix.

The water is chemically acceptable in respect of the parameters measured in general water analyses by Provincial Environmental Chemistry Labs.

Three samples were collected during the sustained pumping test; the analyses are included in the Appendix.

The water is essentially calcium bicarbonate type and similar to the water pumped from the Test-Production well 70 feet away.

As in the earlier Test-Production well, the chloride concentration is higher than normal background and showed a reduction over the pumping period from 79 to 59 mg/l. Sodium concentration is relatively low at 8 mg/l suggesting that the chloride ions may be present as the calcium salt. Dust inhibitors were being used on the access road to Site 2 about 800 feet south of the well. This may account for the chloride concentrations and if so it reflects very high permeability.

The major ions remained reasonably constant in concentration throughout the pumping period, however, fluctuations are noted in the concentration of iron and zinc.

The iron concentration apparently increased from 0.02 to 0.17 mg/l over the 72 hours. This is still within acceptable limits but indicates a trend of increasing concentration with time.

Zinc showed an increase in concentration from 0.01 to 2.1 mg/l. This is a somewhat remarkable increase in a relatively uncommon ion. If the reported concentration is not an analytical error, the metal concentration may reflect influences introduced through the adjacent railway line at a distance of about 150 feet from the well. The acceptable concentration of zinc in drinking water is recommended to be 5.0 mg/l.

Additional monitoring of metal concentrations should be considered with extended pumping.

ADDITIONAL WORKS

In completion of the groundwater testing programme the test holes and wells have been left in the following state:

Site 3. The test hole at this location was plugged and abandoned by backfilling the hole from the bottom (265') to the foot of the 6" casing (175') using locally available gravel. A cement plug was then spotted between 160-175 at the foot of the 6" casing.

A cement plug was placed outside the 6 inch casing extending around the well for several feet at present ground level and sealing the annular space between the 6" and 8" casings at ground level. The 6" casing was plugged from 4 to 15 feet.

SITE 1. The test production well (8" diameter) at this site was capped using a sanitary well seal with all vents plugged. This well is capable of flowing at an estimated 25 igpm. It may represent a potential local source of good quality groundwater and for this reason has not been plugged and abandoned.

The observation well at Site 1 has been capped in a similar fashion but has a 2" elbow and nipple installed with a hand valve. This well has represented a source of water to occupants of the industrial park and tourists in the nearby trailer parks during the summer of 1978. Local users remark on the acceptability of the water. This well is also capable of flowing at about 25 igpm. no significant reduction in flowing head was noted during the summer of 1978.

A wooden cabinet 2½ feet by 2½ feet X 4 feet was erected on a cement slab to cover the overburden observation well at Site 1 and a Stevens type F water level recorder installed in it.

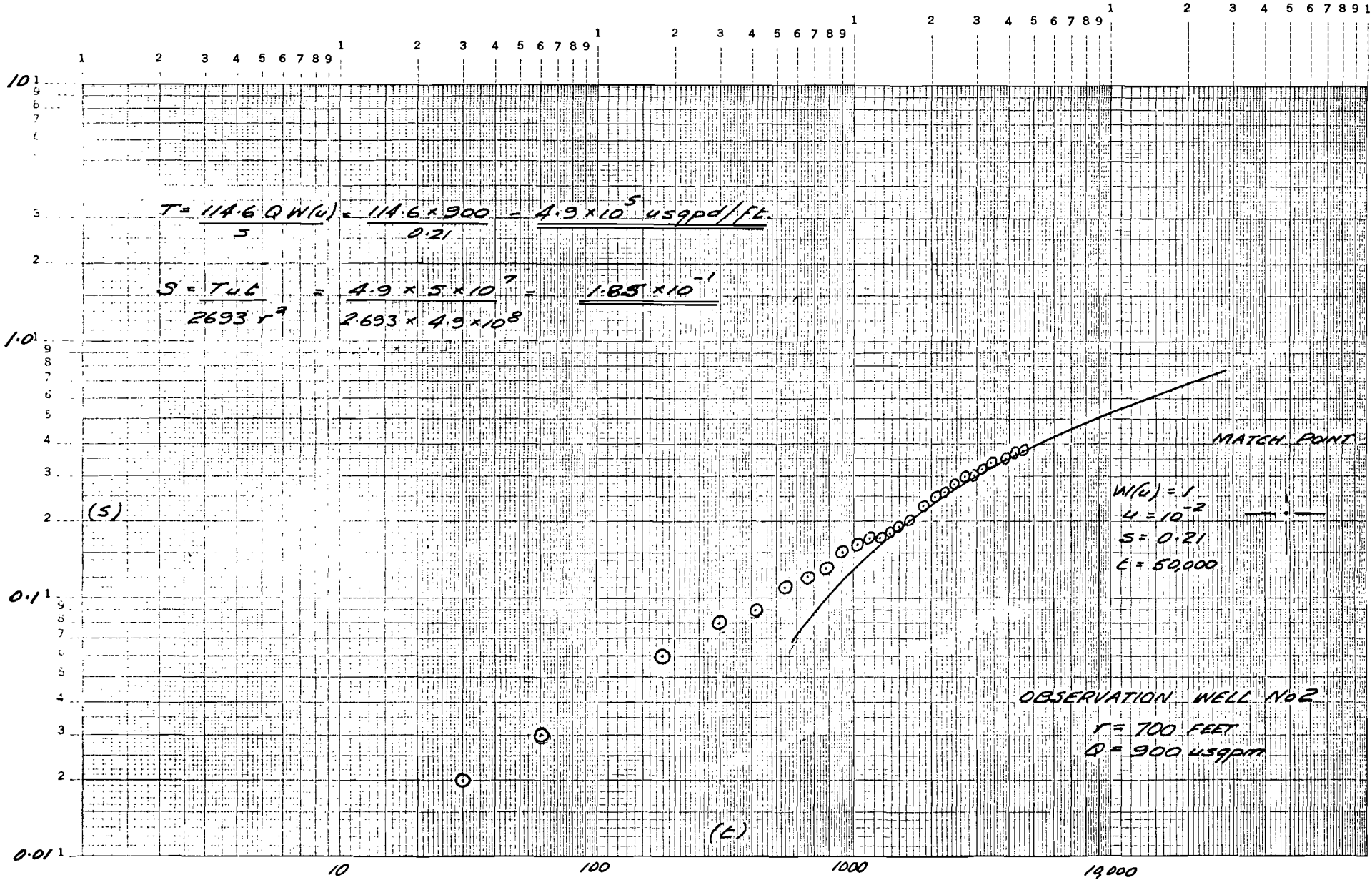
This instrument has an 8 day clock drive (the most useful drive for monitoring the recent pumping test) and a 4" float.

CONCLUSIONS & RECOMMENDATIONS

1. The 12" well at Site 2 is efficiently constructed for pumping rates up to 750 igpm.
2. A well pump designed to produce 400 usgpm (333 igpm) should have a setting of at least 40 feet below present ground level and should preferably be set at 75 feet. The maximum lift in the well is estimated to be 35 feet at 333 igpm.
3. Water quality is acceptable. Routine analyses of quality should be considered at six month intervals to monitor possible changes in quality. Specifically, chlorides and a metal scan to include iron and zinc are recommended.
4. The sensitivity of this aquifer to infiltration contamination must be recognized and all practical measures considered to protect the region of the well field. Protection measures were outlined in a previous report (Phase 2 dated November 1977). To these should be added a cautionary note regarding the railway line immediately north of the Site 2 wells. The rail transport agencies should be advised of the presence of this well field in order that their activities can be controlled as closely as is practical regarding dumping and accidental spillages. Communication avenues should be prepared so that in the event of an emergency the rail agencies can inform those responsible for maintenance of the well field. Immediate response to an emergency spill situation is mandatory in order to protect this highly permeable aquifer.
5. The instrumented observation well at Site 1 should be maintained conscientiously to assemble a hydrograph of groundwater levels.
The agency responsible for maintenance of the AVRIP water supply should arrange to modify the existing instrument to permit it to operate unattended for a period of 30 days for convenient records. The existing instrument requires that it be attended every 8 days.
The assembled hydrographs should be submitted to Environment Nova Scotia for review and if necessary analysis in order to predict incipient danger which might result from reversals of existing groundwater gradients.
Environment Nova Scotia should be encouraged to include the maintenance of this instrument as part of its network of observation wells in the Province, if this is acceptable to that Department.

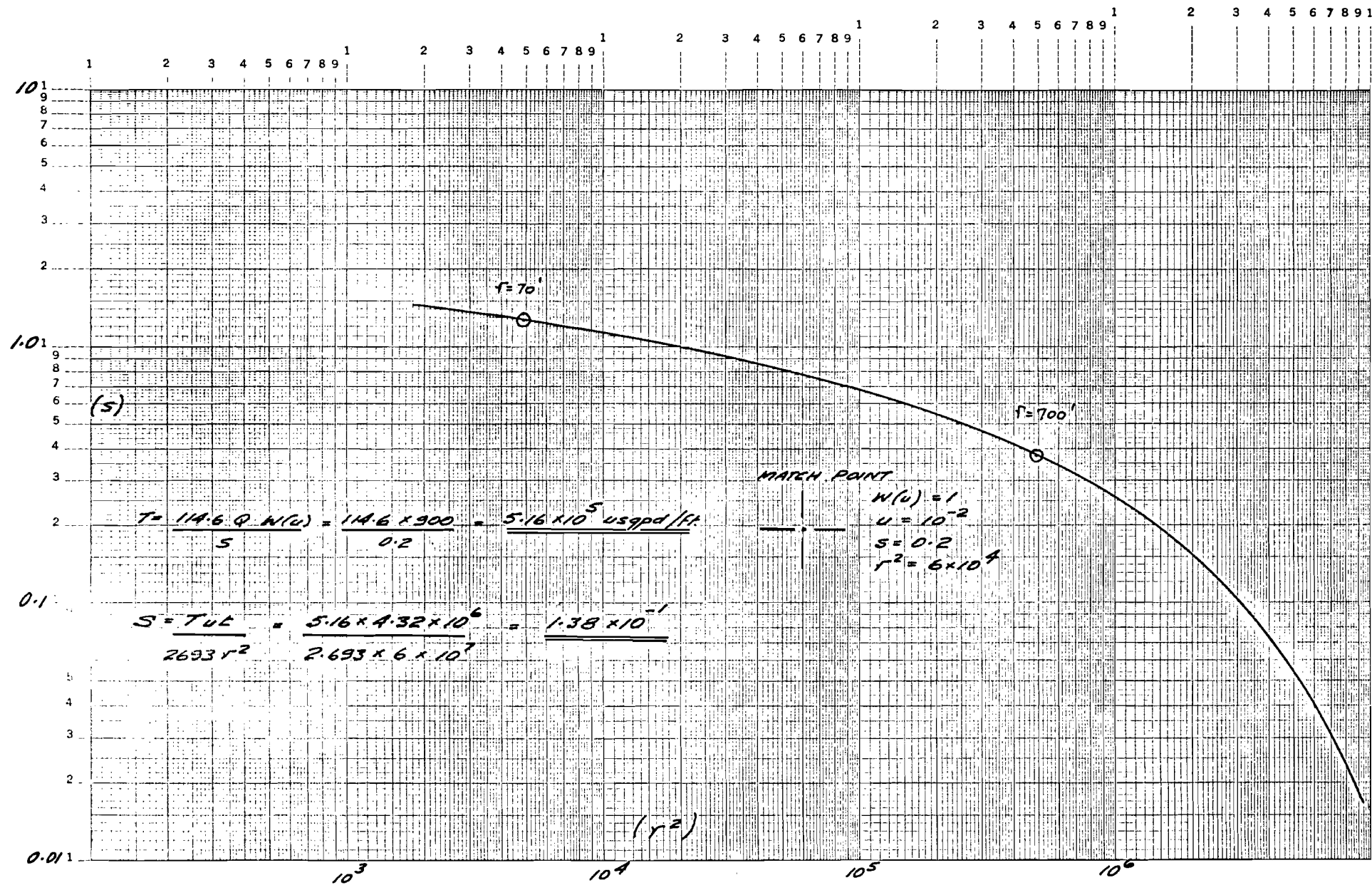
Conclusions & Recommendations (cont'd)

6. When a period of history of pumping at 400 usgpm (333 igpm) is available from the AVRIP, consideration might be given to additional exploitation of the aquifer. Anticipated additional groundwater demand should provide the justification and funding required for a detailed hydro-geological analysis of the aquifer and its relationship to surface water regimes in the region. Factors involved in such a study include detailed topographic data, careful inventory of natural discharges, careful stream gauging in the water courses immediately north of the Site 2 well field and subsurface information to the north of the Site 2 well field. The 12" well is capable of pumping efficiently at rates up to 750 igpm even though it will be equipped initially with a 333 igpm pump. In order to use the well to its greatest capacity, the agency responsible for operating the well field should make application to Water Planning and Management Branch, Environment Nova Scotia, for an increase in the present allowable license (333 igpm).
7. An arrangement should be made to purchase or control the property containing the Site 1 Obs. Well. The bedrock test-production well at Site 1 is a potential source of 150 gpm good quality groundwater, it has been capped rather than abandoned; the agency responsible for water maintenance may wish to control this property also.



AVRIP - KENTVILLE PUMPING TEST, 12" WELL, AUGUST 23-26, 1978
 TIME-DRAWDOWN DATA,

FIGURE 3



AVRIP - KENTVILLE PUMPING TEST, 12" WELL, AUGUST 23-26, 1978

DISTANCE-DRAWDOWN RELATIONSHIP

L = 4320 minutes

FIGURE 4

ENVIRONMENTAL CHEMISTRY
 Division of Clinical Chemistry
 5788 University Ave., Hfx., N. S. B3H 1V8
 Phone XXXXXXXXXX 428-3466

Report To: _____

OWNER AVRIP KENTVILLE

LOCATION PUMPING TEST 30 MINS

SOURCE WELL

GRID _____

DATE TAKEN AUG 23/78

COLLECTED BY D.M. CALLAN

SAMPLE # 78075

Bill To: _____
D.M. CALLAN
800 WINDMILL RD #120
DARTMOUTH, N.S.

DETERMINATION	RESULT		DETERMINATION	RESULT	
	mg/litre	meq/litre			
Sodium	8.5		Total Dissolved Solids	212	mg/litre
Potassium	2.1		Suspended Solids	1.0	mg/litre
Calcium	40		Colour	10	T.C.U.
Magnesium	5.8		Turbidity	1.5	J.T.U.
Hardness (as CaCO ₃)	125		Conductivity	350	umho/cm
Alkalinity (as CaCO ₃)	28		pH	6.8	UNITS
Sulfate	2.0				
Chloride	79				
Fluoride	< 0.1				
Silica, reactive	12				
Phosphate, ortho	0.14				
Nitrate + Nitrite (as N)	0.9		Total Organic Carbon		mg/litre
Ammonia (as N)	< 0.1		Humic Acids		mg/litre
Arsenic	< 0.005				
Metal Scan (See attached)					
Iron	< 0.02				
Manganese	< 0.01		Mercury		mg/litre
Lead	< 0.005				
Copper	< 0.01				
Zinc	< 0.01				

FIELD DATA

Temp: _____

pH: _____

Iron: _____

Remarks:

GENERAL ANALYSIS

DATE _____

Rec'd AUG 28/78

Comp'd Sept 20/78

O. Bergal
 CHEMIST

ENVIRONMENTAL CHEMISTRY
 Division of Clinical Chemistry
 5788 University Ave., Hfx., N. S. B3H 1V8
 Phone [REDACTED] 428-3466

Report To: _____

OWNER AVRIP KENTVILLE
 LOCATION PUMPING TEST 24 HRS
 SOURCE WELL
 GRID _____
 DATE TAKEN AUG 24/78
 COLLECTED BY D.M. CALLAN
 SAMPLE # 78076

Bill To: _____
D.M. CALLAN
800 WINDMILL RD #120
DARTMOUTH, N.S.

DETERMINATION	RESULT		DETERMINATION	RESULT
	mg/litre	meq/litre		
Sodium	8.0		Total Dissolved Solids	178 mg/litre
Potassium	1.8		Suspended Solids	1.3 mg/litre
Calcium	33		Colour	5 T.C.U.
Magnesium	4.7		Turbidity	3.4 J.T.U.
Hardness (as CaCO ₃)	102		Conductivity	260 umho/cm
Alkalinity (as CaCO ₃)	26		pH	7.8 UNITS
Sulfate	2.8			
Chloride	62			
Fluoride	< 0.1			
Silica, reactive	11			
Phosphate, ortho	0.24			
Nitrate + Nitrite (as N)	1.0		Total Organic Carbon	mg/litre
Ammonia (as N)	< 0.1		Humic Acids	mg/litre
Arsenic	< 0.005			
Metal Scan (See attached)				
Iron	< 0.02			
Manganese	0.01		Mercury	mg/litre
Lead	0.005			
Copper	< 0.01			
Zinc	0.14			

FIELD DATA
 Temp: _____
 pH: _____
 Iron: _____

Remarks:
GENERAL ANALYSIS

DATE _____
 Rec'd AUG 28/78
 Comp'd Sept 21/78
O. Bergal.
 CHEMIST

ENVIRONMENTAL CHEMISTRY
 Division of Clinical Chemistry
 5788 University Ave., Hfx., N. S. B3H 1V8
 Phone XXXXXXXXXX 428-3466

Report To: _____

OWNER AVRIP KENTVILLE
 LOCATION PUMPING TEST 72 HRS
 SOURCE WELL
 GRID _____
 DATE TAKEN AUG 26/78
 COLLECTED BY D.M. CALLAN
 SAMPLE # 78077

Bill To: _____
D.M. CALLAN
800 WINDMILL RD #120
DARTMOUTH, N.S.

DETERMINATION	RESULT		✓	DETERMINATION	RESULT	
	mg/litre	meq/litre				
Sodium	8.0			Total Dissolved Solids	166	mg/litre
Potassium	1.8			Suspended Solids	0.8	mg/litre
Calcium	31			Colour	5	T.C.U.
Magnesium	4.5			Turbidity	0.17	J.T.U.
Hardness (as CaCO ₃)	96			Conductivity	265	umho/cm
Alkalinity (as CaCO ₃)	24			pH	6.8	UNITS
Sulfate	1.8					
Chloride	59					
Fluoride	<0.1					
Silica, reactive	11					
Phosphate, ortho	0.23					
Nitrate + Nitrite (as N)	0.9			Total Organic Carbon		mg/litre
Ammonia (as N)	<0.1			Humic Acids		mg/litre
Arsenic	<0.005					
Metal Scan (See attached)						
iron	0.17					
Manganese	0.02			Mercury		mg/litre
Lead	<0.005					
Copper	0.02					
Zinc	2.1					

FIELD DATA
 Temp: _____
 pH: _____
 Iron: _____

Remarks:
GENERAL ANALYSIS

DATE _____
 Rec'd AUG 28/78
 Comp'd Sept 21/78
O. Bergal
 CHEMIST

DATE, TIME	REMARKS	ELAPSED TIME	PUMPED WELL	OBS. WELL No 1 $r = 70'$	OBS. WELL No 2 $r = 700'$
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August 23, 1978

9:30 am	1 SWL		27.16	26.96	18.31
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STEP 1

9:31	($Q = 450 \text{ usgpm}$)	1	34.25	7.09	27.16	0.20
9:32		2	33.75	6.59	27.16	0.20
9:33		3	33.79	6.63	27.16	0.20
9:34		4	33.79	6.63	27.16	0.20
9:35		5	33.79	6.63	27.16	0.20
9:36		6	33.79	6.63	27.16	0.20
9:37		7	33.79	6.63	27.16	0.20
9:38		8	33.79	6.63	27.20	0.24
9:39		9	33.79	6.63	27.20	0.24
9:40		10			27.20	0.24
9:42		12				
9:44		14	33.79	6.63	27.25	0.29
9:46		16			27.08	
9:48		18	33.79	6.63	27.20	0.24
9:50		20	33.79			
9:55		25	33.79	6.63	27.20	0.24
10:00		30	33.79	6.63	27.20	0.24

suspected error.

STEP 2

($Q = 650 \text{ usgpm}$)

10:01		1	37.37	10.21	27.16	0.20
10:02		2	37.50	10.34	27.25	0.29
10:03		3	37.54	10.38		
10:04		4	37.54	10.38	27.25	0.29
10:05		5	37.54	10.38	27.25	0.29
10:06		6			27.27	0.31
10:07		7	37.58	10.42	27.29	0.33
10:08		8			27.29	0.33
10:09		9			27.29	0.33
10:10		10	37.58	10.42	27.29	0.33
10:12		12			27.29	0.33
10:14		14			27.29	0.33
10:16		16	37.58	10.42	27.29	0.33
10:18		18			27.29	0.33
10:20		20	37.58	10.42	27.29	0.33

AVRIP - KENTVILLE
PUMPING TESTS, 12" WELL, SITE 2, August 23-26, 1978.

DATE & TIME	REMARKS	1		2		3		4		5	
		ELAPSED TIME	PUMPED WELL	OBS. WELL No 1	OBS. WELL No 2	OBS. WELL No 1	OBS. WELL No 2	OBS. WELL No 1	OBS. WELL No 2	OBS. WELL No 1	OBS. WELL No 2
10:30	<u>STEP 3</u> (Q = 300 usgpm)	30	37.58	10.42	27.29	0.33	18.31				
10:31		1	41.85	14.69	27.29	0.33	18.31	0			
10:31:30		1.5	42.06	14.90	27.29	0.33					
10:32		2	42.12	14.96	27.29	0.33					
10:32:30		2.5	42.14	14.98	27.42	0.46					
10:33		3	42.17	15.01	27.42	0.46					
10:34		4	42.21	15.05	27.42	0.46					
10:35		5	42.21	15.05							
		6									
		7	42.23	15.07							
		8									
		9									
10:40		10	42.25	15.09							
		12									
		14									
10:45		15	42.31	15.15							
		17	42.33	15.17							
10:50		20	42.35	15.19	27.42	0.46					
		25	42.42	15.26	27.44	0.48					
11:00		30	42.44	15.28	27.46	0.50	18.33	0.02			
11:05		35	42.44	15.28	27.46	0.50					
11:10		40	42.46	15.30	27.48	0.52					
11:15		45	42.48	15.32	27.50	0.54					
11:20		50	42.50	15.34	27.50	0.54					
11:25		55	42.52	15.36	27.50	0.54					
11:30		60	42.52	15.36	27.50	0.54	18.34	0.03			
11:40		70	42.52	15.36	27.50	0.54					
11:50		80	42.52	15.36	27.50	0.54					
12:10		100	42.54	15.38	27.50	0.54					
12:20		110	42.62	15.46	27.52	0.56					
12:40		130	42.62	15.46	27.54	0.58					
1:00		150	42.65	15.49	27.56	0.60					
1:30		180	42.65	15.49	27.58	0.62	18.37	0.06			
2:30		240	42.65	15.49	27.58	0.62					
3:30		300	42.65	15.49	27.60	0.64	18.39	0.08			
4:30		360	42.65	15.49	27.60	0.64					
5:30		420	42.65	15.49	27.62	0.66	18.40	0.09			
6:00		480	42.65	15.49	27.62	0.66					

AVRIP - KENTVILLE
 PUMPING TESTS, 12" WELL, SITE 2, August 23-26, 1978.
 SUSTAINED TEST

DATE, TIME	REMARKS	ELAPSED TIME	PUMPED WELL	OBS. WELL		(Corrected drawdowns)
				No 1 r=70'	No 2 r=700'	
August 23, 1978						
7:30 pm.		540	42.65	15.49	27.62	0.66 18.42 0.11
8:30		600	42.68	15.52	27.64	0.68
9:30		660	42.68	15.52	27.64	0.68 18.43 0.12
10:30		720	42.68	15.52	27.66	0.70
11:30		780	42.68	15.52	27.66	0.70 18.44 0.13
12:30		840	42.72	15.56	27.68	0.72
1:30	Aug. 24th.	900	42.72	15.56	27.70	0.74 18.46 0.15
3:30		1020	42.76	15.60	27.72	0.76 18.47 0.16
5:30		1140	42.76	15.60	27.79	0.81 18.48 0.17
7:30		1260	42.76	15.60	27.83	0.85 18.50 0.19
9:30		1380	42.76	15.60	27.87	0.89 18.51 0.18
11:30		1500	42.75	15.59	27.87	0.89 18.52 0.19
1:30		1620			27.89	0.91 18.53 0.20
3:30		1740			27.90	0.92 18.54 0.21
5:30		1860	42.75	15.59	27.96	0.98 18.56 0.23
7:30		1980			28.00	1.02 18.57 0.24
9:30		2100			28.02	1.04 18.58 0.25
11:30		2220			28.04	1.06 18.59 0.26
1:30	Aug. 25th.	2340			28.06	1.08 18.60 0.27
3:30		2460			28.08	1.10 18.61 0.28
5:30		2580			28.08	1.08 18.62 0.29
7:30		2700			28.10	1.10 18.63 0.30
9:30		2820			28.12	1.12 18.64 0.29
11:30		2940	42.75	15.59	28.12	1.12 18.65 0.30
1:30		3060			28.14	1.14
3:30		3180			28.14	1.14 18.67 0.32
5:30		3300			28.20	1.20 18.68 0.33
7:30		3420			28.25	1.25 18.69 0.34
9:30		3540	42.83	15.67	28.27	1.27
11:30		3660	42.85	15.69	28.27	1.25 18.70 0.33
1:30	Aug. 26th.	3780			28.29	1.27 18.71 0.34
3:30		3900			28.29	1.27 18.72 0.35
5:30		4020	42.87	15.71	28.29	1.27 18.73 0.36
7:30		4140			28.29	1.27 18.74 0.37
9:30		4260			28.29	1.27
10:30	PUMP OFF.	4320	42.85	15.69	28.29	1.27 18.75 0.38

(Page 4 of 4)

AVRIP - KENTVILLE
 PUMPING TESTS, 12" WELL, SITE 2
 RECOVERY
 Aug 23-26, 1978

DATE TIME	ELAPSED TIME	PUMPED WELL		OBS WELL No 1	OBS. WELL No 2
10:30 Aug 26/78		42.56		28.33	
10:30:15	0.25	28.83			
10:30:30	0.50	28.42	0.41		
10:30:45	0.75	28.29	0.54		
10:31	1	28.25	0.58	28.21	0.12
32	2	28.17	0.66	28.02	0.31
33	3	28.12	0.71	27.87	0.46
34	4	28.12	0.71	27.98	0.35
35	5	28.12	0.71	27.98	0.35
36	6	28.10	0.73	27.98	0.35
37	7	28.08	0.75	27.94	0.39
38	8	28.08	0.75		
39	9	28.08	0.75	27.92	0.41
40	10	28.06	0.77	27.92	0.41
42	12			27.89	0.44
44	14	28.06	0.77	27.89	0.44
46	16			27.94	0.39
48	18			27.94	0.39
50	20			27.87	0.46
55	25	28.02	0.81	27.87	0.46
11:00	30	28.00	0.83	27.83	0.50
05	35	27.98	0.85	27.81	0.52
15	45	27.96	0.87	27.81	0.52
30	60	27.98	0.85	27.81	0.52
45	75	27.94	0.89	27.77	0.56
12:00	90	27.92	0.91	27.70	0.63
15	105	27.88	0.94		
12:30 Aug 27/78	1560	27.50	1.33		
	Residual dd	0.34			

SEE
 STEVENS
 RECORDED
 CHART

CALCULATED INTERFERENCE DRAWDOWNS

MATCH POINT DATA:

Assume: $T = 5 \times 10^5$ usgpd/ft.

$S = 0.1$

$W(u) = 1$

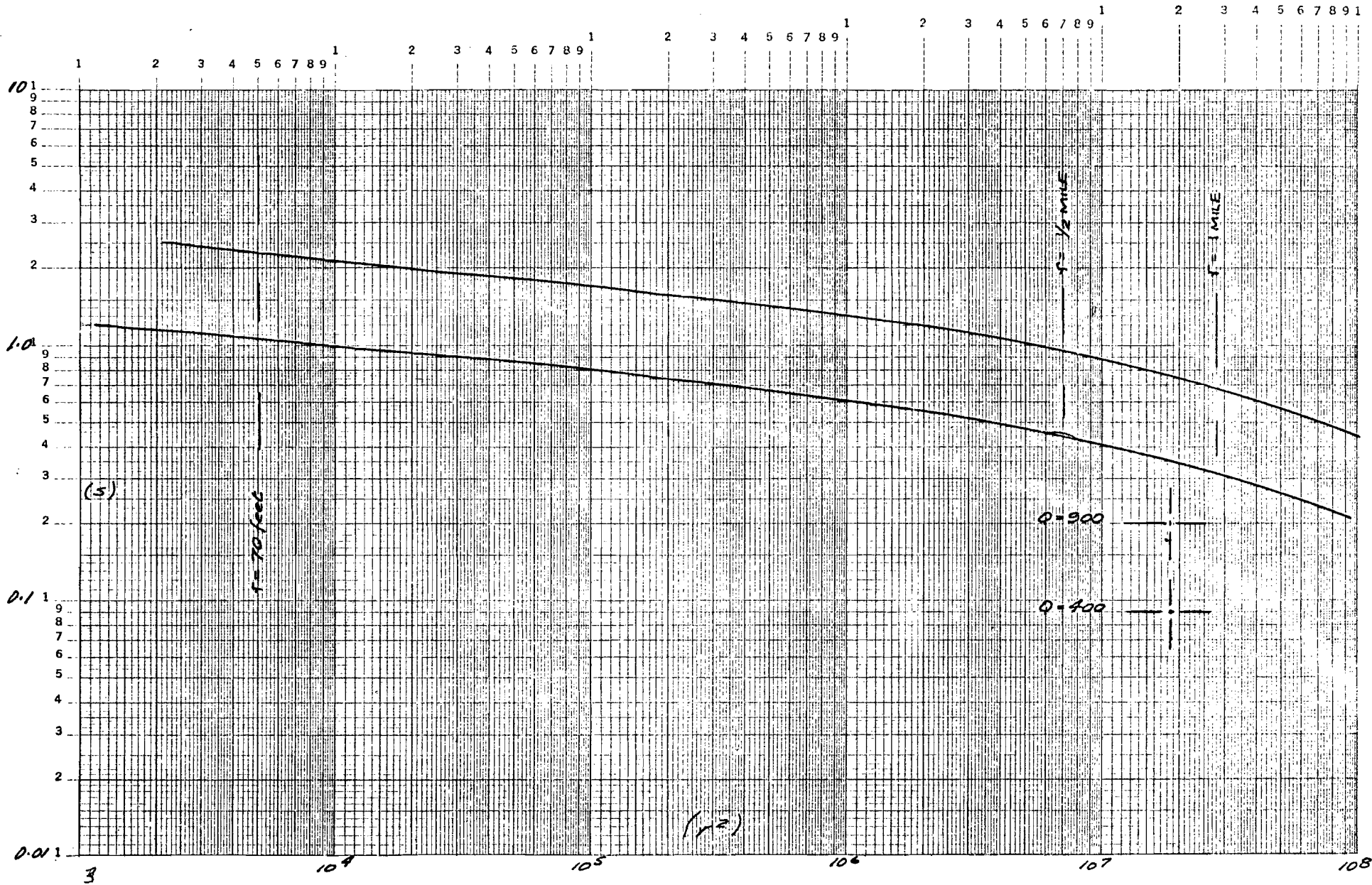
$u = 10^{-2}$

$t = 10^6$ minutes (2 YEARS)

$$s = \frac{114.6 Q W(u)}{T} \quad \text{for } Q = 400 \text{ usgpm} = \frac{114.6 \times 400}{5 \times 10^5} = \underline{0.09 \text{ ft}}$$

$$\text{for } Q = 900 \text{ usgpm} = \frac{114.6 \times 900}{5 \times 10^5} = \underline{0.20 \text{ ft}}$$

$$r^2 = \frac{T u t}{2693 S} = \frac{5 \times 10^9}{2693 \times 10^2} = \underline{1.85 \times 10^7}$$



THEORETICAL DRAWDOWN IN AIRIP-KENTVILLE AQUIFER
 $T = 2 \text{ YEARS } (1 \times 10^6 \text{ min})$

AM